

Lecture 4  
18/04/19

④ Vertical alignment

Gradient

- Ruling gradient
- Limiting gradient
- Exceptional gradient
- Min gradient

Vertical curve

- Summit curve
- Valley curve

Gradient - Slope due to gradual rise or gradual fall in the dirt of vehicle movement is called gradient

- If it is gradual rise there is a slope is called ascending gradient / rising gradient / positive gradient.

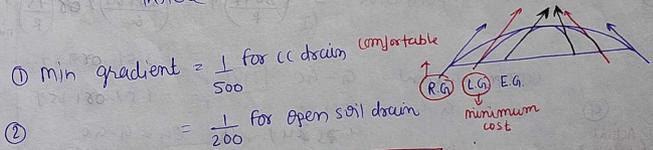
- If it is gradual fall then the slope is called descending gradient / falling gradient and (-ve) gradient.

① R.G → this is the max gradient which designer want to provide to a vertical provide of road. It depend upon design speed and topography

② L.G → this gradient is steeper than ruling gradient and it is provided only when cost of construction by L.G is less than the cost of construction by R.G.

③ E.G → this gradient is steeper than above other gradient and it is provided only when case is unavoidable

④ M.G → this gradient is provided for drainage point of view in order to provide gravity flow of water inside the drainage system



$E.G > L.G > R.G > M.G$

# Maximum gradient as per IRC →

	R.G.	L.G.	E.G.
① Plain & rolling terrain	3.3%	5%	6.7%
② Mountainous & steep terrain having elevation more than 3000m above MSL	5%	6%	7%
③ Mountainous & steep terrain having elevation upto 3000m above MSL	6%	7%	8%

Note - { At higher elevation lesser gradient is provided bcoz availability of oxygen is less at higher elevation which may reduce pulling power of vehicle }

# Grade compensation when there is a horizontal curve in addition to gradient there is a loss of traction due to curve and gradient both. In this case IRC suggest to compensate to grade so that pulling power of vehicle should not be compromise.

$$\text{Gradient} - \text{grade compensation} = \text{compensated grade} \quad ***$$

$$\text{Grade compensation} = \left(\frac{30+R}{R}\right)\% \quad \text{or} \quad \left(\frac{75}{R}\right)\% \quad \text{min}$$

● As per IRC

- Grade compensation is not require for a gradient flatter than 4%.
- Compensated gradient should not be less than 4%.

(Pb) (54)  $\text{Grade compensation} = \left(\frac{30+R}{R}\right)\% = \left(\frac{30+60}{60}\right)\% \text{ or } \left(\frac{75}{R}\right)\% \text{ min}$

$$\text{compe gradient} = G - G_c = 1.5\% - 0.875\% = 0.625\%$$

(Q)  $G_c = G - G_c = 6 - 1.25 = 4.75\%$   
 Actual grade compensation =  $5 - 4 = 1\%$

(A) Vertical curve

● summit curve:

are the vertical curve with convexity upward or convexity downward. It can be formed by 2 gradient in following way.

Case (1) (+w) to (+w)



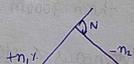
change in grade:  $N = |m_1 - m_2|$

Case (2) (+w) to flat



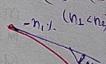
change in gradient:  $N = |m_1 - 0|$

Case (3) (+w) to (-w)



$N = |m_1 - (-m_2)|$   
 $N = |m_1 + m_2|$

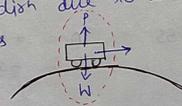
Case (4) (-w) to (-w)



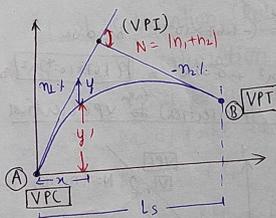
$N = |(-m_1) - (-m_2)|$   
 $N = |m_2 - m_1|$

● Some important point:

- ① convexity upward and convexity downward
- ② Vertical point of intersection (VPI) always lies above the curve.
- ③ Summit curve and design only for sight distance criteria
- ④ Summit curve are design with square Parabola shape due to best riding quality and due to simplicity of calculation.
- ⑤ Ideal shape of summit curve is circular.
- ⑥ Generally there is no discomfort in summit curve b/c centrifugal force act in upward dir due to which effective weight of wheel reduces.



● Derivation



1) General eq of summit curve

$$y = ax^2 + bx + c$$

At Point A  $x=0, y=0$

$$0 = a \cdot 0^2 + b \cdot 0 + c$$

$$c = 0$$

$$\left[\frac{dy}{dx} = 2ax + b\right] = \text{Gradient}$$

At Point A

$$\frac{dy}{dx} = +m_1, x=0$$

$$[+m_1 = 2a \cdot 0 + b]$$

$$b = +m_1$$

At Point B

$$\frac{dy}{dx} = -m_2, x=Ls$$

$$-m_2 = 2a \cdot Ls + m_1$$

$$a = -\left[\frac{m_1 + m_2}{2Ls}\right] = -\frac{N}{2Ls}$$

$$a = -\frac{N}{2Ls}$$

$$y = \left[-\frac{N}{2Ls}\right]x^2 + m_1x$$

② \* \* \* <sup>2013</sup> Position of summit point / crest point / highest point from VPC

$$\frac{dy'}{dx} = 0, \quad 2a + b = 0, \quad x = -\frac{b}{2a} = -\frac{n_1}{2\left(-\frac{N}{2L_s}\right)}$$

$$x = \frac{n_1 \cdot L_s}{N}$$

③ Apex eqn

$$\text{temp} \Rightarrow n_1 = \frac{y+y'}{x}$$

$$\Rightarrow y + y' = n_1 x$$

$$\Rightarrow y = n_1 x - y' = n_1 x - \left[ \left(-\frac{N}{2L_s}\right)x^2 + n_1 x \right]$$

$$y = \left[ \frac{N}{2L_s} \right] x^2$$

④ 55  $\left. \begin{matrix} n_1 = +0.02 \\ n_2 = -0.03 \end{matrix} \right\} N = |n_1 + n_2| = |0.02 + 0.03| = 10.05$

$$x = \left( \frac{n_1 \cdot L_s}{N} \right) = \frac{0.02}{0.05} \times 150 = 60\text{m}$$

then  $a = \frac{-N}{2L_s} = \frac{-10.05}{2 \times 150} = (-1.67 \times 10^{-4}), \quad b = 0.02$

$$y = (-1.67 \times 10^{-4})x^2 + 0.02x = \boxed{(1.67 \times 10^{-4})x^2} \text{ Ans}$$

# Position of VPI (Vertical Point of Intersection) <sup>from</sup> VPC (Vertical Point of Curve)

from  $\Delta AVE$

$$n_1 = \frac{VE}{x} = VE = n_1 x$$

from  $\Delta VDB$

$$n_2 = \frac{VD}{L_s - x} \Rightarrow VD = n_2 L_s - n_2 x$$

$$\Rightarrow y'_{\text{at } x=L_s} = DE = VE - VD$$

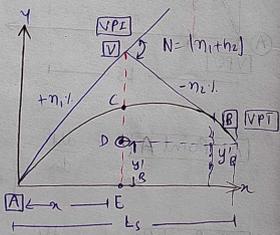
$$\left[ -\frac{N}{2L_s} \right] L_s^2 + n_1 L_s = n_1 x - n_2 L_s + n_2 x$$

$$-\frac{NL_s}{2} + n_1 L_s + n_2 L_s = n_1 x + n_2 x$$

$$-\frac{NL_s}{2} + NL_s = n_1 x + n_2 x$$

$$\frac{NL_s}{2} = Nx$$

$$x = \frac{L_s}{2} \text{ ***}$$



Case-1 When the curve is symmetrical  $\rightarrow (N_1 = N_2)$

- ① In this case vertical line passing VPI will bisect length of curve into equal half.
- ② this line is always pass through summit point of curve.
- ③ In this case RL of starting point and end point exactly same.

Case-2 When the curve is nt symmetrical  $(N_1 \neq N_2)$

- ① In this case vertical line passing through VPI will bisect length of curve in two equal half.
- ② this line will never pass through summit point of curve.
- ③ In this case the curve will be tilted and RL of starting point and end point will be different.

RL of calculation (Reduce level)

a) RL of calculation.

if RL of A is known

a) RL of Point just below (VPI)

$$RL = RL_A + y' \text{ @ } x = \frac{L_s}{2}$$

$$= RL_A + \left( -\frac{N}{2L_s} \right) \left( \frac{L_s^2}{4} \right) + n_1 \frac{L_s}{2}$$

$$RL = RL_A + n_1 \left( \frac{L_s}{2} \right) - \frac{NL_s}{8} = RL_A + n_1 L_s - \frac{n_2 L_s}{2} - \frac{n_1 L_s}{2}$$

b) RL of end point

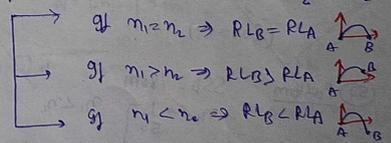
$$RL_B = RL_A + y' \text{ @ } x=L_s$$

$$= RL_A + \left( -\frac{N}{2L_s} \right) L_s^2 + n_2 L_s$$

$$= RL_A + \left( -\frac{NL_s}{2} \right) + n_2 L_s$$

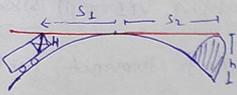
$$= RL_A + n_2 L_s - \frac{L_s}{2} (n_1 + n_2)$$

$$RL_B = RL_A + \frac{L_s}{2} (n_1 - n_2)$$



# length of summit curve  $\rightarrow$

Case-① When  $L_s > SD$



$H$  = height of driver eye  
 $h$  = Height of obstruction

$$L_s = \frac{NS^2}{2(\sqrt{H} + \sqrt{h})^2}$$

Standard Relation by IRC

For SSD  $H=1.2m, h=0.15m$

$$L_s = \frac{NS^2}{2(\sqrt{1.2} + \sqrt{0.15})^2} = \frac{NS^2}{4.39}$$

$$L_s = \frac{NS^2}{4.4}$$

For OSD/ISD  $H=1.2m, h=1.1m$

$$L_s = \frac{NS^2}{2(\sqrt{1.2} + \sqrt{1.1})^2} = \frac{NS^2}{9.6}$$

$$L_s = \frac{NS^2}{9.6}$$

Case-② When  $L_s < SD$

$$L_s = 2S - \frac{2(\sqrt{H} + \sqrt{h})^2}{N}$$

IRC Relations

1] For SSD

$$L_s = 2S - \frac{4.4}{N}$$

2] For OSD/ISD

$$L_s = 2S - \frac{9.6}{N}$$

NOTE

For calcul of sight distance in design of summit curve valley curve effect of gradient should not be taken

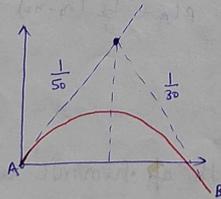
$$S = 0.278 V t_p + \frac{V^2}{254f} \quad ***$$

WB.

46  $n = \frac{L_s}{2}$   
 $(200 = L_s)$

57 Problem

SSD = 180m  
 $n_1 < n_2$   
 $n_1 = +\frac{1}{50}$   
 $n_2 = -\frac{1}{30}$   
 $RL_A = 100m$



change in gradient

$$N = \left| \frac{1}{50} + \frac{1}{30} \right| = \frac{4}{75}$$

Assume  $L_s > SD$   $L_s = \frac{NS^2}{4.4} = \frac{\left(\frac{4 \times 180^2}{75}\right)}{4.4} = 392.72m > 180m$   
 (SSD)  
 Assumption was safe

① RL of point just below VPI

$$RL_c = RL_A + n_1 \frac{L_s}{2} - \frac{N L_s^2}{8}$$

$$= 100 + \frac{1}{50} \times \frac{392.72}{2} - \frac{4/75 \times 392.72^2}{8} = 101.31m$$

② RL of summit point

$$x = \left(\frac{n_1}{N} L_s\right) = \left(\frac{\frac{1}{50} \times 392.72}{4/75}\right) = 147.27m$$

$$y @ x = 147.27$$

$$y @ x = 147.27 = \left[\frac{-N}{2L_s}\right] x^2 + n_1 x$$

$$= \left[\frac{-\frac{4}{75}}{2 \times 392.72}\right] \times 147.27^2 + \frac{1}{50} \times 147.27$$

$$= 1.47m$$

$$RL_{summit\ point} = RL_A + y @ x = summit$$

$$= 100 + 1.47 = 101.47m$$

Valley curve

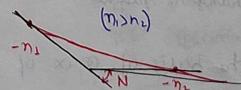
are vertical curve with concavity upward and convexity downward.

- it can be formed by 2 gradients in following ways

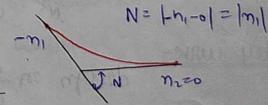
① (-ve) to (+ve)

② (+ve) to flat

③ (-ve) to (+ve)

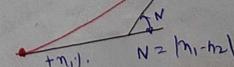


$$N = \frac{|-n_2 - (-n_1)|}{1} = |n_1 - n_2|$$



① (+ve) to (+ve)

$$N = |n_1 - n_2|$$



$$N = \frac{|-n_2 - (-n_1)|}{1} = |n_1 - n_2|$$

Some important point

- ① concavity upward
- ② convexity downward
- ③ V.P.I always lies below the curve
- ④ valley curve are design taking following consideration into account.

⑤ ⑥ Comfort to passengers

⑦ Head-light sight distance\*\*\*

⑧ Aesthetic of valley curve

Note H LSD (max. distance visible through head light of vehicle is called HSD)

generally  $HSD \approx SSD$

⑦ Valley curve are made fully transitional by adding 2 similar cubic parabola transition curve.

⑧ Centrifugal force act in downward dir due to which effective wt of vehicle increases which may cause jerking/ discomfort to passengers

⑨ Position of lowest point of valley curve from (V.C)

$$x = \frac{L_v}{2} \left( \frac{N_1}{N_1 + N_2} \right)^{\frac{1}{3}}$$

$L_v$  = length of valley curve

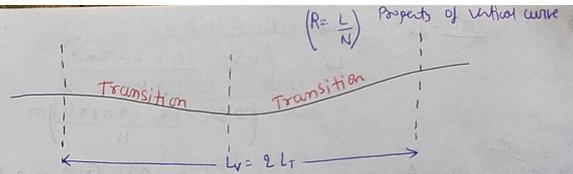
$N_1$  = 1st gradient

$N_2$  = change in gradient

# length of valley curve - design on the basis of max. of

following 2 criteria

criteria ① As per comfort con<sup>t</sup>



length of transition curve

$$L_T = \frac{V^3}{CR} = \frac{V^3}{C \cdot \left( \frac{L_T}{N} \right)} = \sqrt{\frac{NV^3}{C}} \quad \boxed{L_T = \sqrt{\frac{NV^3}{C}}}$$

Length of valley curve

$$L_v = 2L_T$$

$$\boxed{L_v = 2 \sqrt{\frac{NV^3}{C}}}$$

As per IRC

$C = 0.6 \text{ m/s}^3$  for cubic parabola T.C

$$L_v = 2 \sqrt{\frac{N(0.278V)^3}{0.6}}$$

$$= 0.378 \sqrt{NV^3}$$

As per IRC  $\boxed{L_v = 0.38 \sqrt{NV^3}} \rightarrow \text{kmph}$

Don't use formula

$$C = \frac{80}{75+V}$$

It is valid for only spiral T.C

# Criteria ② As per HSD

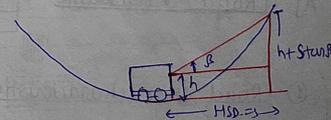
Case-① When  $L_v > HSD$

$h$  = Height of Headlight

$\beta$  = Deam angle

$$L_v = \frac{NS^2}{2h + 2S \tan \beta} \text{ (m)}$$

$$L_v = \frac{NS^2}{1.5 + 0.035S} \text{ m} \quad (h = 0.75 \text{ m and } \beta = 2^\circ)$$



Case-II

When  $L_v < HSD$

$$L_v = \left( 2S - \frac{2h + 2.5 \tan \beta}{N} \right)$$

$$= \left( 2S - \frac{1.5 + 0.035(S)}{N} \right) m$$

Problem

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Criteria 1

As per comfort

$$L_v = 0.36 \sqrt{NV^3} = 0.36 \sqrt{\frac{11.0 \times 26^3}{50}}$$

$$= 73.63 m$$

Criteria 2

As per HSD

Assume  $L_v > HSD$

$$L_v = \frac{N S^2}{1.5 + 0.035(S)} = \frac{11 \times (27.5)^2}{1.5 + 0.03 \times 27.5} = 200.11 m$$

$$HSD = SSD = \left( 0.278 \times 80 \times 2.5 + \frac{(80)^2}{254 \times 0.35} \right)$$

$$= 127.59 m$$

$$L_v = \begin{cases} 73.63 \\ 200.11 \end{cases}$$

max

$$L_v = 200.11 m$$

## CHAPTER 3

### TRAFFIC ENGINEERING

- Traffic characteristic
- Traffic study and analysis
- Traffic control devices and Regulation

A) Traffic characteristic

- Road User characteristic
- Vehicle characteristic
- Booking characteristic

1) Road user characteristic: Factor affecting road user charact.

- Physical: a) vision b) Hearing strength c) Strength d)
- Psychological: a) Attentiveness, b) Fear c) Anger, d) Superstitions e) Impatience f) Mutually

3) Mental → Knowledge, Skill, intelligence, and literacy

$$\text{Vision} = \frac{x}{y} = \left( \frac{\text{Actual Distance}}{\text{Required Distance}} \right)$$

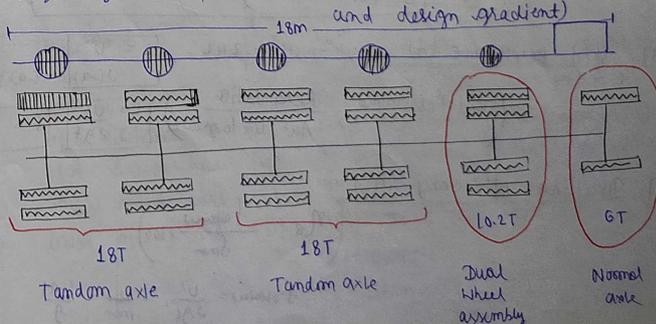
$\frac{6}{5}$  Vision / Normal vision (It is the ability of person vision due to which he can recognised a letter of size 8.5mm at a distance of 6m)

$\frac{6}{5}$  vision (this vision is poorer than the normal vision because he can recognise at a distance of 6m what a normal person can be recognise at a distance of 9m)

# Vehicular characteristic →

Dimension

- Length of vehicle affect extra widening, turning radius, road capacity and parking design. (maximum length 18m)
- Width of vehicle affect lane width, stall shoulder width, parking width (max-2.44)
- Height of vehicle → height of vehicle affect height of bridge, height of electric, telephone, pole, etc (maximum height 4.75m)
- Weight of loaded vehicle (wt affect thickness of pavement and design gradient)



522T

① Power of Vehicle:-

gt effect limiting and permissible value

of gradient

② Speed of vehicle gt effect all geometrical element and traffic control device etc signal rotary

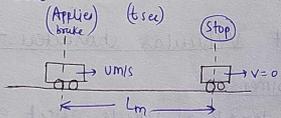
# Braking Test atleast two of the following 3 parameters are needed in braking test to determine skid resistance of pavement.

Initial speed  $U$  m/s, length ( $L_m$ ) (skid distance)

time of brake application =  $t_{sec}$

1] Retardation

$$a = \frac{v - u}{t} \Rightarrow (-a) = \frac{v}{t}$$



(m/s) Retardation =  $\left( \frac{\text{Initial speed}}{\text{time}} \right)$

2] Skid distance

$$L = Ut + \frac{1}{2}at^2$$

$$= Ut + \frac{1}{2} \left( -\frac{U}{t} \right) t^2 = Ut - \frac{1}{2}Ut = \frac{1}{2}Ut$$

$$L = \frac{1}{2}Ut$$

$$\text{Skid distance} = \frac{1}{2} \times \text{initial speed} \times \text{time}$$

3] Skid Resistance (f)

$$v^2 = U^2 + 2aL$$

$$L = \frac{U^2}{2(-a)}$$

$$(-a) = gf$$

(Skid Resistance =  $\frac{\text{Retardation}}{\text{Acc due to gravity}}$ )

$$L = \frac{U^2}{2gf}$$

$$f = \frac{(-a)}{g}$$

4] Braking efficiency ( $\eta_B$ )

$$\eta_B = \frac{f_{\text{obtained}}}{f_{\text{max}}} \times 100$$

$$f_{\text{obtained}} = \frac{U^2}{2gL}, \quad f_{\text{max}} = \frac{(-a)}{g}$$

① Pb  
W.B.

$$V = 50 \text{ km/h}$$

$$L = 16.5 \text{ m}$$

$$f_{\text{aws}} = 0.497$$

$$\eta_B = ?$$

③

$$\eta_B = \frac{f_{\text{obtained}}}{f_{\text{max}}} \times 100 = \frac{0.596}{0.687} \times 100 = 86.75\%$$

$$L = 5.8 \text{ m}, U = 30 \text{ km/h}$$

$$f = \frac{U^2}{2gL} = \left( \frac{0.278 \times 10}{1.8} \right) = 0.17 \text{ m/s}^2$$

$$f = \frac{(-a)}{g} = \frac{6.17}{9.81} = 0.628$$

②  
1.7 m = L  
t = 1.4 sec

$$L = \frac{1}{2}Ut$$

$$7 = \frac{1}{2} \times U \times 1.4 \Rightarrow$$

$$U = 10 \text{ m/s and } f = \frac{U^2}{2gL} = \frac{10^2}{(2 \times 9.81 \times 7)} = 0.726 \text{ m/s}^2$$