A square matrix B is skew symmetric if

1.

# Q. No. 1 – 20 Carry One Mark Each

(A)  $B^{T} = -B$  (B)  $B^{T} = B$  (C)  $B^{-1} = B$  (D)  $B^{-1} = B^{T}$ 

2.	For a scalar function	$f(x, y, z) = x^2 + 3y^2 + 2z^2$	, the gradient at the	e point P (1,2,-1) is
	(A) $2\vec{i} + 6\vec{j} + 4\vec{k}$	(B) $2\vec{i} + 12\vec{j} - 4\vec{k}$	(C) $2\vec{i} + 12\vec{j} + 4\vec{k}$	(D) √56
3.	The analytic function	$f(z) = \frac{z-1}{z^2+1}$ has sing	ularities at	
	•	$z^2 + 1$		
	(A) 1 and -1	(B) 1 and i	(C) 1 and -i	(D)i and -i
4.		drical pressure vessel is subjected to an in		
	(A) 14MPa	(B) 1.4MPa	(C) 0.14MPa	(D)0.014MPa
5.		upture of concrete n (f <sub>ck</sub> ) in MPa according		characteristic cube
	(A) 5000f <sub>ck</sub>	(B) 0.7f <sub>ck</sub>	(C) $5000\sqrt{f_{ck}}$	(D) $0.7\sqrt{f_{ck}}$
6.	In the theory of plas moment is called	stic bending of beams	, the ratio of plasti	c moment to yield
	(A) Shape factor		(B) Plastic section	modulus
	(C) Modulus of resilie	ence	(D) Rigidity modul	
	(-)		(- )5	
7.		collapse, the parti- mating the design stre	-	-
	(A) 1.15 and 1.5	(B) 1.0 and 1.0	(C) 1.5 and 1.15	(D) 1.5 and 1.0
8.	of the external loadir	cross sectional plane ng on the beam has to le cross-section of the	pass through to er	
	(A) Moment centre	(B) Centroid	(C) Shear centre	(D) Elastic centre
9.	The square root of the sectional area is called	ne ratio of moment of	inertia of the cross	section to its cross
	(A) Second moment	of area	(B) Slenderness ra	ntio
	(C) Section modulus		(D) Radius of gyra	tion

10.	Deposit with flocculat (A) Clay particles set (B) Clay particles set (C) Sand particles se (D) Sand particles se	tle on sea bed tle on fresh water lake ttle on river bed		
11.	Dilatancy correction is  (A) Cohesive and sat  (B) Saturated silt/fin correction  (C) Saturated silt/fin correction  (D) Coarse sand un overburden corre	urated and also has Note sand and Note value we sand and Note valued and der dry condition at	Value of SPT > 15 e of SPT < 10 after e of SPT >15 after	er the overburden
12.	A precast concrete pi 1.0m with an efficien combined temporary per Modified Hiley For (A) 3000kN	cy of 0.6. The set val compression of the pi	ue observed is 4mn le, cushion and the sistance of the pile i	n per blow and the ground is 6mm. As
13.	Direct step method of (A) Applicable to non (B) Applicable to pris (C) Applicable to both (D) Not applicable to	-prismatic channels matic channels n prismatic and non-p	rismatic channels	s
14.	The relationship amo of an aquifer is (A) $S_y = S_r + \eta$			
15.	The depth of flow in a Manning's n is 0.018, is (A) 0.713m/s		f the channel as per	Kennedy's method
16.	The reference pressur (A) 20μPa			
17.	Particulate matter (fl fossil fuels are better (A) Cotton bag house (C) Cyclone	removed by	uent gases from the (B) Electrostatic per (D) Wet scrubber	_

18.	The value of lateral fas per India Roads C	riction or side friction ongress guidelines is	used in the design	of horizontal curve
	(A) 0.40	(B) 0.35	(C) 0.24	(D)0.15
19.		the load sustained by The CBR value of the s		pecimen at 5.0mm
	(A) 10.0%	(B) 5.0%	(C) 3.6%	(D) 2.4%
20.	In quadrantal bearing	g system, bearing of a	line varies from	
	(A) 0° to 360°	(B) 0° to 180°	(C) 0° to 90°	(D)0° N to 90°s
	Q. N	lo. 21 – 56 Carry Tw	o Marks Each	
21.		$f(x,y,z) = x^2 + 3y^2 +$ the direction of a vector		al derivative at the
				(5) (5
	(A) -18	(B) −3√6	(C) 3√6	(D) 18
22.	The value of the inte	gral $\int_{C} \frac{\cos(2\pi z)}{(2z-1)(z-3)} dz $ (v	where C is a closed cur	ve given by $ z  = 1$ is
	(A) –πi	(B) $\frac{\pi i}{5}$	(C) $\frac{2\pi i}{5}$	(D) πi
23.	Solution of the difference	ential equation $3y \frac{dy}{dx}$	+ 2x = 0 represents a	a family of
	(A) ellipses	(B) circles	(C) parabolas	(D) hyperbolas
24.	Laplace transform for	the function $f(x) = cc$	osh(ax) is	
	(A) $\frac{a}{s^2 - a^2}$	(B) $\frac{s}{s^2 - a^2}$	(C) $\frac{a}{s^2 + a^2}$	(D) $\frac{s}{s^2 + a^2}$
25.		e following set of linea y + 2z = 34; $4y - 3z = 1$		
	The pivots for elimina	ation of x and y are		
	(A) 10 and 4	(B) 10 and 2	(C) 5 and 4	(D)5 and -4
26.	The standard normal	probability function ca	an be approximated	as
	$F(x_N) = $	1		
	$F(x_N) = \frac{1}{1 + \exp(-1.72)}$	$255x_{N}\left x_{N}\right ^{0.12}\right)$		

Where  $x_N$  = standard normal deviate. If mean and standard deviation of annual precipitation are 102cm and 27cm respectively, the probability that the annual precipitation will be between 90cm and 102cm is

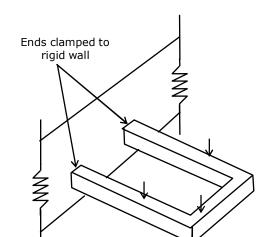
- (A) 66.7%
- (B) 50.0%
- (C) 33.3%
- (D) 16.7%

- 27. Consider the following statements:
  - I. On a principal plane, only normal stress acts
  - II. On a principal plane, both normal and shear stresses act
  - III. On a principal plane, only shear stress acts
  - IV. Isotropic state of stress is independent of frame of reference

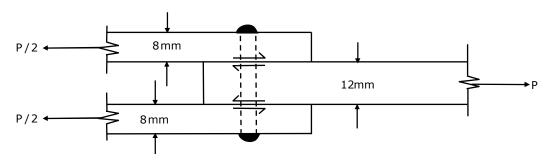
The TRUE statements are

- (A) I and IV
- (B) II
- (C) II and IV
- (D) II and III
- 28. The degree of static indeterminacy of a rigidly jointed frame in a horizontal plane and subjected to vertical loads only, as shown in figure below is





- (B) 4
- (C) 3
- (D) 1
- 29. A 12mm thick plate is connected to two 8mm plates, on either side through a 16mm diameter power driven field rivet as shown in the figure below. Assuming permissible shear stress as 90MPa and permissible bearing stress as 270MPa in the rivet, the rivet value of the joint is



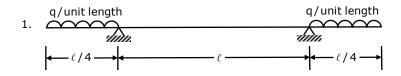
- (A) 56.70kN
- (B) 43.29kN
- (C) 36.19kN
- (D)21.65kN

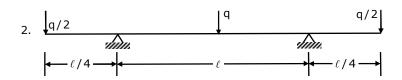
- 30. A hollow circular shaft has an outer diameter of 100mm and a wall thickness of 25mm. The allowable shear stress in the shaft is 125MPa. The maximum torque the shaft can transmit is
  - (A) 46kN m
- (B) 24.5kN m
- (C) 23kN m
- (D) 11.5kN m
- 31. Consider the following statements for a compression member
  - The elastic critical stress in compression increases with decrease in slenderness ratio
  - II. The effective length depends on the boundary conditions at its ends
  - III. The elastic critical stress in compression is independent of the slenderness
  - IV. The ratio of the effective length to its radius of gyration is called as slenderness ratio

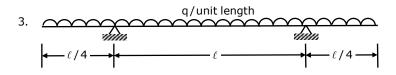
The TRUE statements are

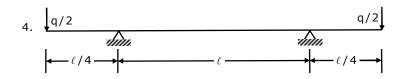
- (A) II and III
- (B) III and IV
- (C) II, III and IV (D)I, II and IV
- 32. **Group I** gives the shear force diagrams and **Group II** gives the diagrams of beams with supports and loading. Match the **Group I** with **Group II**

**Group I** qI/2qI/4qI/2qI/4ql/4 q/2 q/2 q/2 q/2 q/2q/2q/2









(A) P-3,Q-1,R-2,S-4

(B) P-3,Q-4,R-2,S-1

(C) P-2,Q-1,R-4,S-3

- (D) P-2,Q-4,R-3,S-4
- 33. A rectangular concrete beam of width 120mm and depth 200mm is prestressed by pretensioning to a force of 150kN at an eccentricity of 20mm. The cross sectional area of the prestressing steel is  $187.5 \text{mm}^2$ . Take modulus of elasticity of steel and concrete as  $2.1 \times 10^5 \text{MPa}$  and  $3.0 \times 10^4 \text{MPa}$  respectively. The percentage loss of stress in the prestressing steel due to elastic deformation of concrete is
  - (A) 8.75
- (B) 6.125
- (C) 4.81
- (D)2.19
- 34. **Column I** gives a list of test methods for evaluating properties of concrete and **Column II** gives the list of properties

#### Column I

- P. Resonant frequency test
- Q. Rebound hammer test
- R. Split cylinder test
- S. Compacting factor test

#### Column II

- 1. Tensile strength
- 2. Dynamic modulus of elasticity
- 3. Workability
- 4. Compressive strength

The correct match of the test with the property is

(A) P-2,Q-4,R-1,S-3

(B) P-2,Q-1,R-4,S-3

(C) P-2,Q-4,R-3,S-1

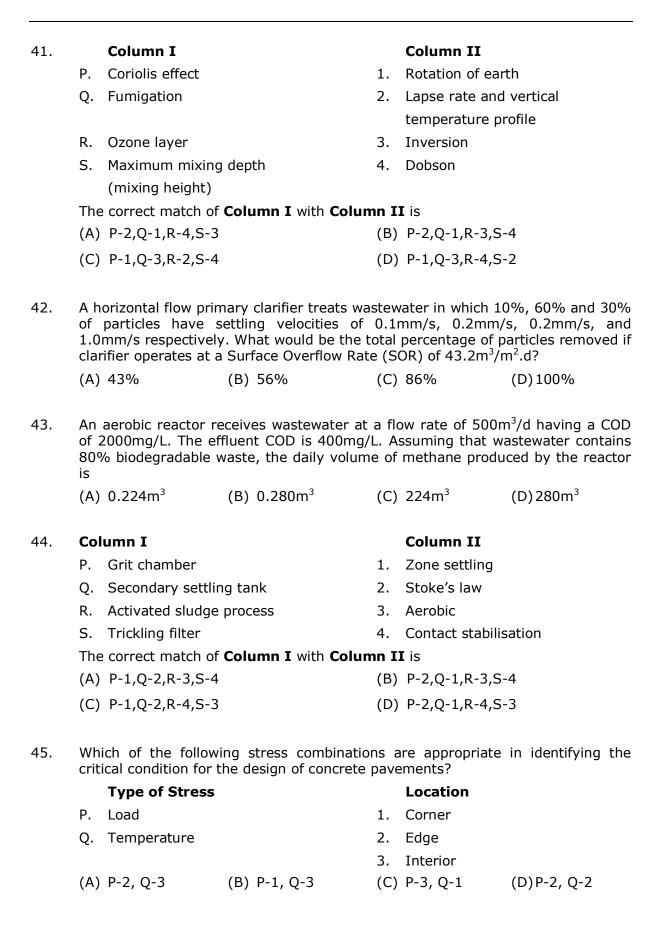
(D) P-4,Q-3,R-1,S-2

	Liquid Limit = 35%			
	Plastic Limit = 27%			
	The soil classification is			
	(A) GM	(B) SM	(C) GC	(D)ML-MI
36.	A plate load test is carried out on a 300mm×300mm plate placed at 2 m below the ground level to determine the bearing capacity of a 2m×2m footing placed at same depth of 2m on a homogeneous sand deposit extending 10m below ground. The ground water table is 3m below the ground level. Which of the following factors <b>does not</b> require a correction to the bearing capacity determined based on the load test?			m footing placed at 10m below ground. ch of the following
	(A) Absence of the o	overburden pressure d	uring the test	
	(B) Size of the plate	is much smaller than	the footing size	
	(C) Influence of the ground water table			
	(D) Settlement is re	corded only over a lim	ited period of one or	two days
37.		n a 100mm diameter sity of water is $1.13 imes$		
	(A) 0.0015	(B) 0.032	(C) 0.037	(D)0.048
38.	A rectangular open The critical depth of	channel of width 4.5m the channel is	n is carrying a disch	arge of 100m³/sec.
	(A) 7.09m	(B) 3.69m	(C) 2.16m	(D)1.31m
39.	Water (v. = 9.879k)	$1/m^3$ ) flows with a flo	ow rate of 0.3m <sup>3</sup> /sec	through a nine AB
33.	Water $(\gamma_w = 9.879 kN/m^3)$ flows with a flow rate of $0.3m^3/sec$ through a pipe Al of 10m length and of uniform cross section. The end 'B' is above end 'A' and th pipe makes an angle of $30^\circ$ to the horizontal. For a pressure of $12kN/m^2$ at th end 'B', the corresponding pressure at the end 'A' is			ove end 'A' and the
	(A) 12.0kN/m <sup>2</sup>	(B) 17.0kN/m <sup>2</sup>	(C) 56.4kN/m <sup>2</sup>	(D)61.4kN/m <sup>2</sup>
40.	period of the crop is	of 437ha is to be in 90 days and the tota of 15cm occurs during	l depth of water req	uired by the crop is
	(A) 437ha/cumec	(B) 486ha/cumec	(C) 741ha/cumec	(D)864ha/cumec

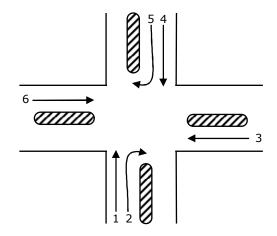
The laboratory test results of a soil sample are given below:

Percentage finer than 4.75 mm = 60 Percentage finer than 0.075 mm = 30

35.



- 46. A rest vertical curve joins two gradients of +3% and -2% for a design speed of 80km/h and the corresponding stopping sight distance of 120m. The height of driver's eye and the object above the road surface are 1.20m and 0.15m respectively. The curve length (which is less than stopping sight distance) to be provided is
  - (A) 120m
- (B) 152m
- (C) 163m
- (D) 240m
- 47. On a specific highway, the speed-density relationship follows the Greenberg's model  $[v=v_f \ln(k_i/k)]$ , where  $v_f$  and  $k_i$  are the free flow speed and jam density respectively. When the highway is operating at capacity, the density obtained as per this model is
  - (A) e.k<sub>i</sub>
- (B)  $k_i$  (C)  $k_i/2$  (D)  $k_i/e$
- 48. A three-phase traffic signal at an intersection is designed for flows shown in the figure below. There are six groups of flows identified by the numbers 1 through 6. Among these 1, 3, 4 and 6 are through flows and, 2 and 5 are right turning. Which phasing scheme is **not feasible**?



Combination choice	Phase I	Phase II	Phase III
Р	1, 4	2, 5	3, 6
Q	1, 2	4, 5	3, 6
R	2, 5	1, 3	4, 6
S	1, 4	2, 6	3, 5
D (D) O		(C) D	(D) C

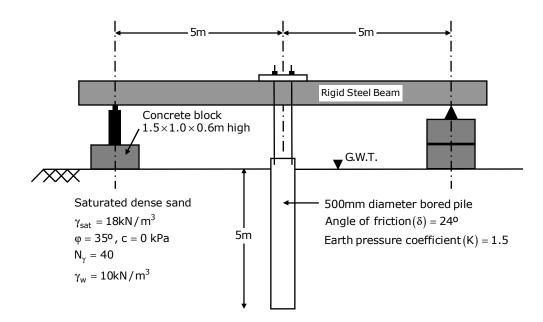
(A) P

- (B) Q
- (C) R
- (D)S
- The magnetic bearing of a line AB was N 59° 30′ W in the year 1967, when the 49. declination was 4° 10' E. If the present declination is 3°W, the whole circle bearing of the line is
  - (A) 299° 20′
- (B) 307° 40′ (C) 293° 20′
- (D)301° 40′

- 50. Determine the correctness or otherwise of the following **Assertion [a]** and the **Reason [r]:** 
  - **Assertion [a]:** Curvature correction must be applied when the sights are long **Reason [r]:** Line of collimation is not a level line but is tangential to the level line
  - (A) Both [a] and [r] are true and [r] is the correct reason for [a]
  - (B) Both [a] and [r] are true but [r] is **not** the correct reason for [a]
  - (C) Both [a] and [r] are false
  - (D) [a] is false but [r] is true

### Common Data Questions: 51 & 52

Examine the test arrangement and the soil properties given below



- 51. The maximum pressure that can be applied with a factor of safety of 3 through the concrete block, ensuring no bearing capacity failure in soil using Terzaghi's bearing capacity equation without considering the shape factor, depth factor and inclination factor is
  - (A) 26.67kPa
- (B) 60kPa
- (C) 90kPa
- (D) 120kPa
- 52. The maximum resistance offered by the soil through skin friction while pulling out the pile from the ground is
  - (A) 104.9kN
- (B) 209.8kN
- (C) 236kN
- (D)472kN

### Common Data Questions: 53 & 54

Following chemical species were reported for water sample from a well:

Specials	Concentration (milli equivalent/L)
Chloride (Cl <sup>-</sup> )	15
Sulphate (SO <sub>4</sub> <sup>2-</sup> )	15
Carbonate (CO <sub>3</sub> <sup>2-</sup> )	05
Bicarbonate (HCO <sub>3</sub> -)	30
Calcium (Ca <sup>2+</sup> )	12
Magnesium (Mg <sup>2+</sup> )	18
pH	8.5

- 53. Total hardness in mg/L as CaCO<sub>3</sub> is
  - (A) 1500
- (B) 2000
- (C) 3000
- (D)5000
- 54. Alkalinity present in the water in mg/L as CaCO<sub>3</sub> is
  - (A) 250
- (B) 1500
- (C) 1750
- (D)5000

#### Common Data Questions: 55 & 56

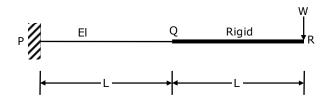
One hour triangular unit hydrograph of a watershed has the peak discharge of 60m<sup>3</sup>/sec.cm at 10hours and time base of 30 hours. The  $\phi$  index is 0.4cm per hour and base flow is 15m<sup>3</sup>m/sec

- 55. The catchment area of the watershed is
  - (A) 3.24km<sup>2</sup>
- (B) 32.4km<sup>2</sup>
- (C)  $324 \text{km}^2$  (D)  $3240 \text{km}^2$
- If these is rainfall of 5.4cm in 1 hour, the ordinate of the flood hydrograph at 15<sup>th</sup> 56. hour is
  - (A) 225 m<sup>3</sup>/sec
- (B)  $240 \text{ m}^3/\text{sec}$  (C)  $249 \text{ m}^3/\text{sec}$  (D)  $258 \text{ m}^3/\text{sec}$

#### Linked Answer Questions: Q.57 to Q.60 Carry Two Marks Each

# Statement for Linked Answer Questions: 57 & 58

In the cantilever beam PQR shown in figure below, the segment PQ has flexural rigidity EI and the segment QR has infinite flexural rigidity



- 57. The deflection and slope of the beam at 'Q' are respectively
  - (A)  $\frac{5WL^3}{6FI}$  and  $\frac{3WL^2}{2FI}$  (B)  $\frac{WL^3}{3FI}$  and  $\frac{WL^2}{2FI}$  (C)  $\frac{WL^3}{2FI}$  and  $\frac{WL^2}{FI}$  (D)  $\frac{WL^3}{3EI}$  and  $\frac{3WL^2}{2EI}$

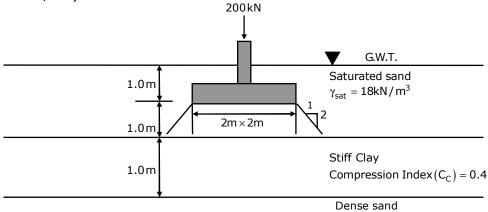
- The deflection of the beam at 'R' is 58.
  - (A)  $\frac{8WL^3}{FI}$
- (B)  $\frac{5WL^3}{6FI}$
- (C)  $\frac{7WL^3}{3EI}$  (D)  $\frac{8WL^3}{6EI}$

# Statement for Linked Answer Questions: 59 & 60

- 59. A saturated undisturbed sample from a clay strata has moisture content of 22.22% and specific weight of 2.7. Assuming  $\gamma_w = 10 \text{kN/m}^3$ , the void ratio and the saturated unit weight of the clay, respectively are
  - (A)  $0.6 \text{ and } 16.875 \text{kN/m}^3$

(B) 0.3 and 20.625 kN/m<sup>3</sup>

- (C) 0.6 and 20.625kN/m<sup>3</sup>
- (D)  $0.3 \text{ and } 16.975 \text{ kN}/\text{m}^3$
- 60. Using the properties of the clay layer derived from the above question, the consolidation settlement of the same clay layer under a square footing (neglecting its self weight) with additional data shown in the figure below (assume the stress distribution as 1H:2V from the edge of the footing and  $\gamma_w = 10kN/m^3$ ) is



- (A) 32.78mm
- (B) 61.75mm
- (C) 79.5mm
- (D) 131.13mm